

1.GENERAL NOTES

A.Contractor shall be responsible for checking and coordinating all dimensions Civil/Site, Fire Protection, and other project drawings. In case of conflict, the Engineer shall be notified and shall resolve the conflict.

B.Contractor shall be solely responsible for the correctness of dimensions and quantities and for the fitting to other work; for work to be confirmed and correlated at the site; and for the coordination of this work with all other trades. The Engineer's review of Contractor's submissions does not relieve Contractor from these responsibilities.

C.Means methods, techniques, sequences, and procedures of construction as well as compliance with OSHA and other Federal, State, or Local safety laws and regulations is the exclusive responsibility of the Contractor, their Subcontractor(s), Suppliers, Consultants, and/or Servants.

2.STRUCTURAL DESIGN CRITERIA

A.Referenced Codes:
i. International Building Code 2015

ii. ACSE 7—10 Minimum Design Loads for Buildings and Other Structures iii. ACI 318—14 Building Code Requirements for Structural Concrete

B.Design Loads: i. Dead Loads

56,000 lb a.Weight of Tank b.Weight of LPG @ 85% density (4.24 lb/gal) 108,120 lb ii.Live Loads: N/A iii.Roof Snow Loads: N/A iv.Wind Design Data: a.Basic Wind Speed (V) 126 mph b.Wind Load Importance Factor (I) 1.0 c.Wind Exposure d.Topographic Factor (Kzt) 1.0 v.Earthquake Design Data: a.Seismic Importance Factor (I) b.Short Period Spectral Response (Ss) c.Long Period Spectral Response (S1) 0.058 d.Site Class e.Spectral Response Coefficient (Sds) 0.213 f.Spectral Response Coefficient (Sd1) 0.093 g.Seismic Design Category Horizontal Saddle Supported h.Basic Seismic Resisting System Steel Vessel V=0.30*Sds*W*I=10,487 i.Design Base Shear

3.FOUNDATION

A.The design of the foundation is based upon assumed soil conditions.
Contractor shall expose and verify the capacity of the existing bearing material.
B.All footings shall be founded on natural undisturbed material or upon compacted structural fill having a minimum safe bearing capacity of 3,000 psf.
Structural fill shall be placed over the natural undisturbed material in maximum 8—inch lifts compacted to 95% of maximum Modified Proctor dry density.
C.Where suitable undisturbed material is found higher or lower than shown on

Rigid Nonbuilding Structure

j.Response modification Factor (R)

k.Analysis Procedure Used

the plans isolated footings may be lowered or raised and piers, added, increased, or reduced in height with prior review and approval by the Engineer.

D.Contractor shall safeguard all excavations from freezing, rain, ground water. No foundations shall be placed in water or upon frozen ground.

E. All foundations and piers shall be centered under members supported and edges of footings shall not be placed a greater than a 1:2 (vertical to horizontal) slope with respect to any adjacent footings, unless otherwise noted.
F. Exterior footings shall bear at or below a frost depth of 4'-0" below finished grade.

G.Contractor is solely responsible for temporary sheeting, shoring, or bracing required to safely excavate for foundation work.H.Backfilling against foundation walls shall not be performed until concrete has

H.Backfilling against foundation walls shall not be performed until concrete has attained sufficient strength. Both sides of piers shall be backfilled simultaneously.

4.CONCRETE

A.All concrete shall be mixed, placed, cured, and tested in accordance with ACI 318 and ACI 301 except that provisions of the specification prevail where more

B.Contractor shall submit mix designs in accordance with ACI 318 Section 5.3 "Proportioning on the basis of experience and/or trial mixtures" for each type of concrete for review and approval by the Architect and Structural Engineer prior to performing concrete work. Each concrete mix shall utilize the largest practicable nominal maximum course aggregate in accordance with ACI 318 Section 3.3. Use of calcium chloride containing aggregates or admixtures is not permitted.

C.Concrete shall be air entrained (5%-7%), normal weight (145 pcf) with a 4,000 psi minimum compressive strength at 28 days.

D.Concrete slump shall not exceed 4 inches unless a high-range water reducing

D.Concrete slump shall not exceed 4 inches unless a high—range water reduci admixture is utilized, where the maximum slump may be increased to 8 inches.

E.All reinforcing steel shall be deformed bars conforming to ASTM A615 (Grade 60) unless otherwise noted. Bar sizes, nominal bar diameters, and nominal cross—sectional areas shall conform to ACI 318.

F. Detailing of reinforcing shall conform to ACI Pub. SP-66, ACI Detailing Manual.

G.Minimum concrete cover shall be provided for reinforcement in accordance with ACI 318, unless otherwise noted. Contractor shall not increase cover as a means to reduce development lengths, Ld, of bars unless approved by

H.Splicing of reinforcement is permitted only at locations shown on the structural drawings or as accepted. Where splicing is utilized, splices shall be tension lap splices developing the full tensile capacity of the reinforcement.

Minimum lap for reinforcement bars shall be per ACI 318 but no less than 12 inches.

Welding of reinforcement is not acceptable unless specified on the structural drawings or submitted and approved by the Engineer.

I. All concrete construction shall be cast monolithically without horizontal

construction joints, unless otherwise shown on the structural drawings.

J.Contractor is responsible for proper and adequate shoring of all concrete work including form work, ties, reinforcing chairs, standees, etc.

5.<u>SHOP_DRAWIN</u>

A.Shop drawings shall be submitted for structural steel and reinforcing steel.
 B.Drawings shall show all lengths, connections, sizes, welds, etc. and relation of members to affected construction trades.

C.Structural steel connections shall be designed for the loads show on plan and in the Structural Steel Notes.D.Deviations from plan details shall be clearly shown in submittal package.

D.Deviations from plan details shall be clearly shown in submittal package.
 E.Shop drawings will be checked for general design features only. Review does not cover dimensions, quantities, accuracy fit, and adequacy of details which are solely the responsibility of the Contractor. SFC's review is limited to those items included in our design scope.

6.STRUCTURAL TESTS AND INSPECTIONS

A.Structural Tests, Inspections, and Reports for concrete construction, soils, foundations, and other applicable construction shall be promptly submitted to the Structural Engineer of Record (Engineer), Building Official, and Contractor.

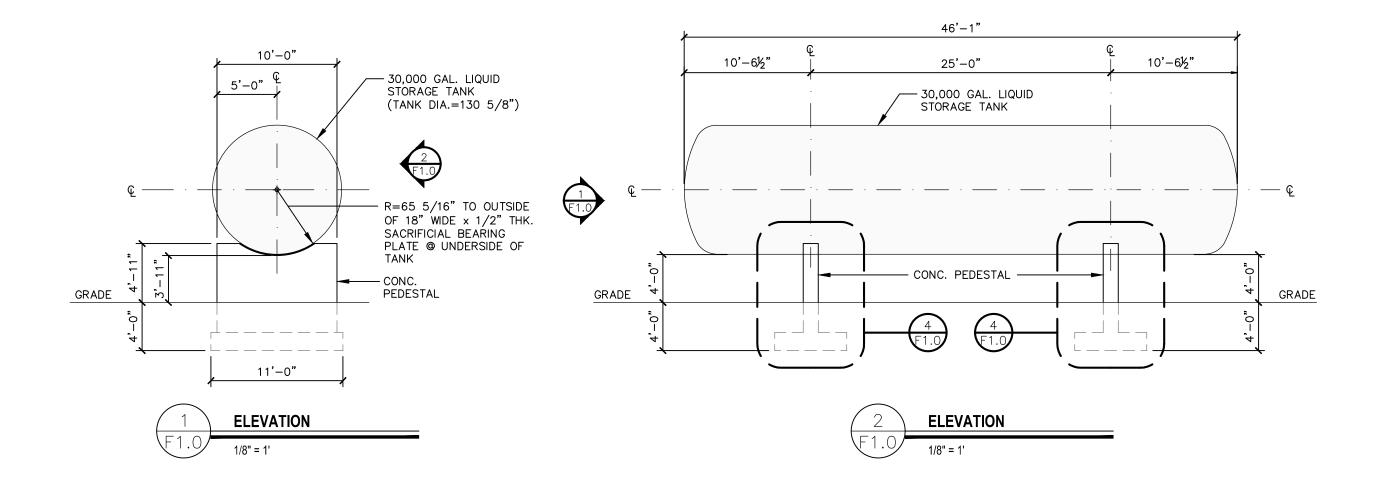
the Structural Engineer of Record (Engineer), Building Official, and Contractor.

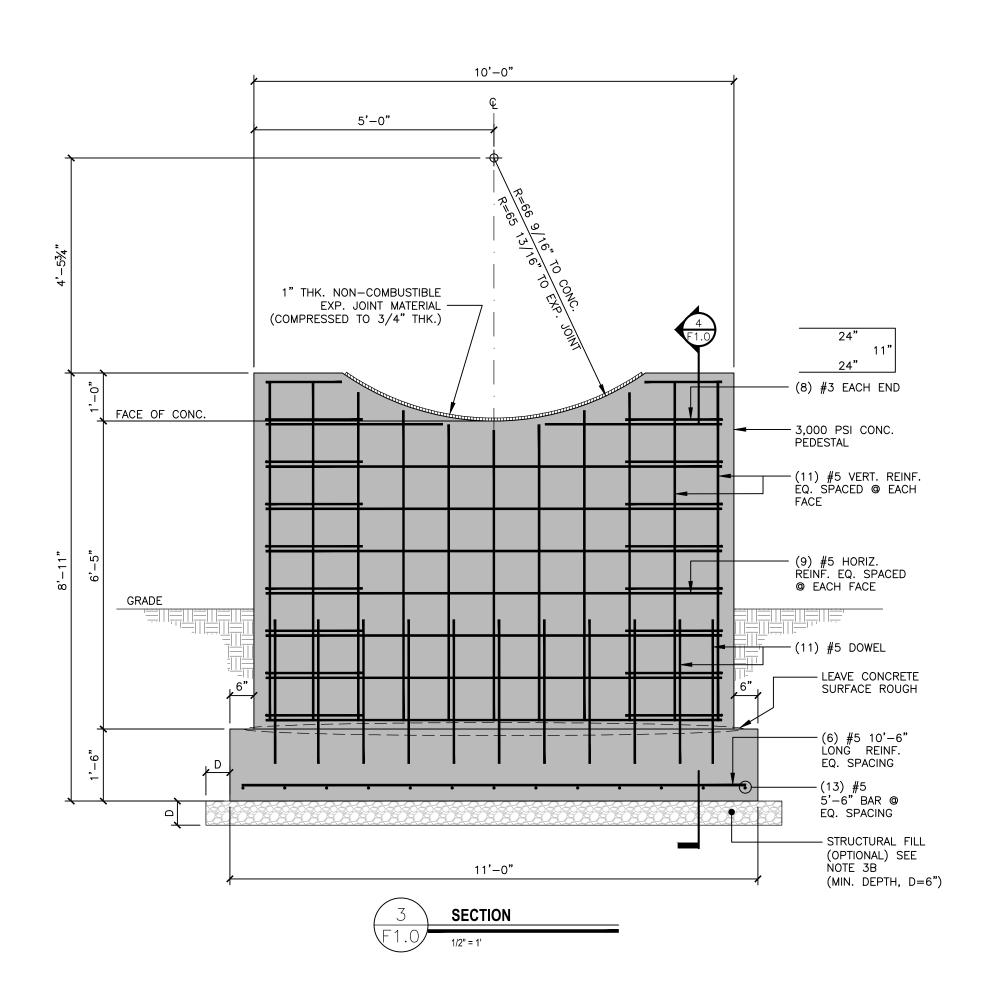
B.Tests and Inspections shall be completed in accordance with IBC 2015. Refer to Statement of Special Inspections & Quality Assurance plan issued with final construction documents for the required program of special inspections for each building material/system.

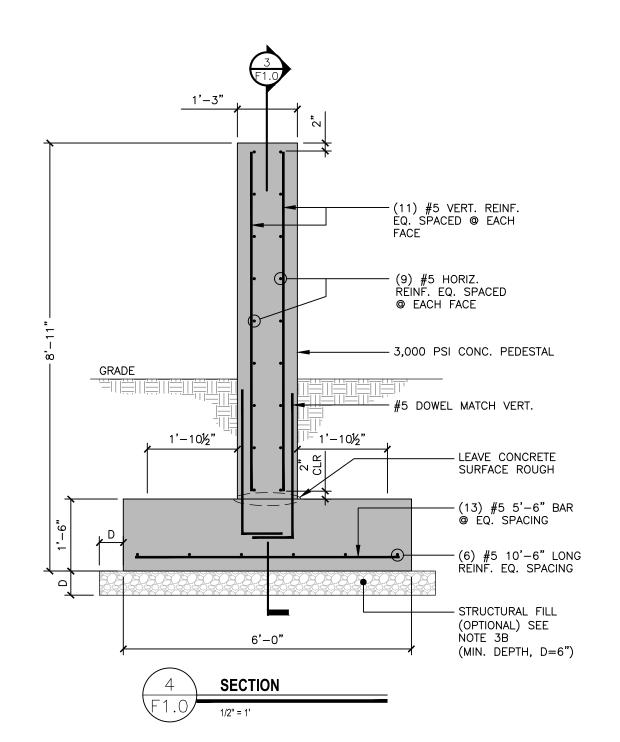
C.Discrepancies shall be brought to the immediate attention of the Contractor for correction. If they are not corrected, discrepancies shall be brought to the attention of the Engineer and Building Official prior to completion of that

D.Where tests indicate work does not comply with specified requirements the work shall be removed and replaced, or otherwise repaired per the Engineer's approval. Additional testing and inspection, at the Contractor's expense, will be performed to determine compliance of the replaced or repaired work with the specified requirements.

E.Structural Observations: Contractor shall notify Engineer of progress of construction for coordination of site observations per IBC 2015. These observations are for review of general conditions only.









JEFFREY M.

CIVIL

ENGINEERING

183 ROCKINGHAM RD UNIT 3 EAST
WINDHAM, NH 03087
(603) 647-8700

www.sfceng.com

Scale: AS SHOWN

Sutton, IVIA ank Foundation Design Dr

Project No

Crown Energy 10 Rocky Ridge Road Windham, NH 03087

F1.0

DWG NO.

© SFC ENGINEERING PARTNERSHIP INC. 2023

CROWN DILEO GAS SUTTON MA TANK FOUNDATION 666040



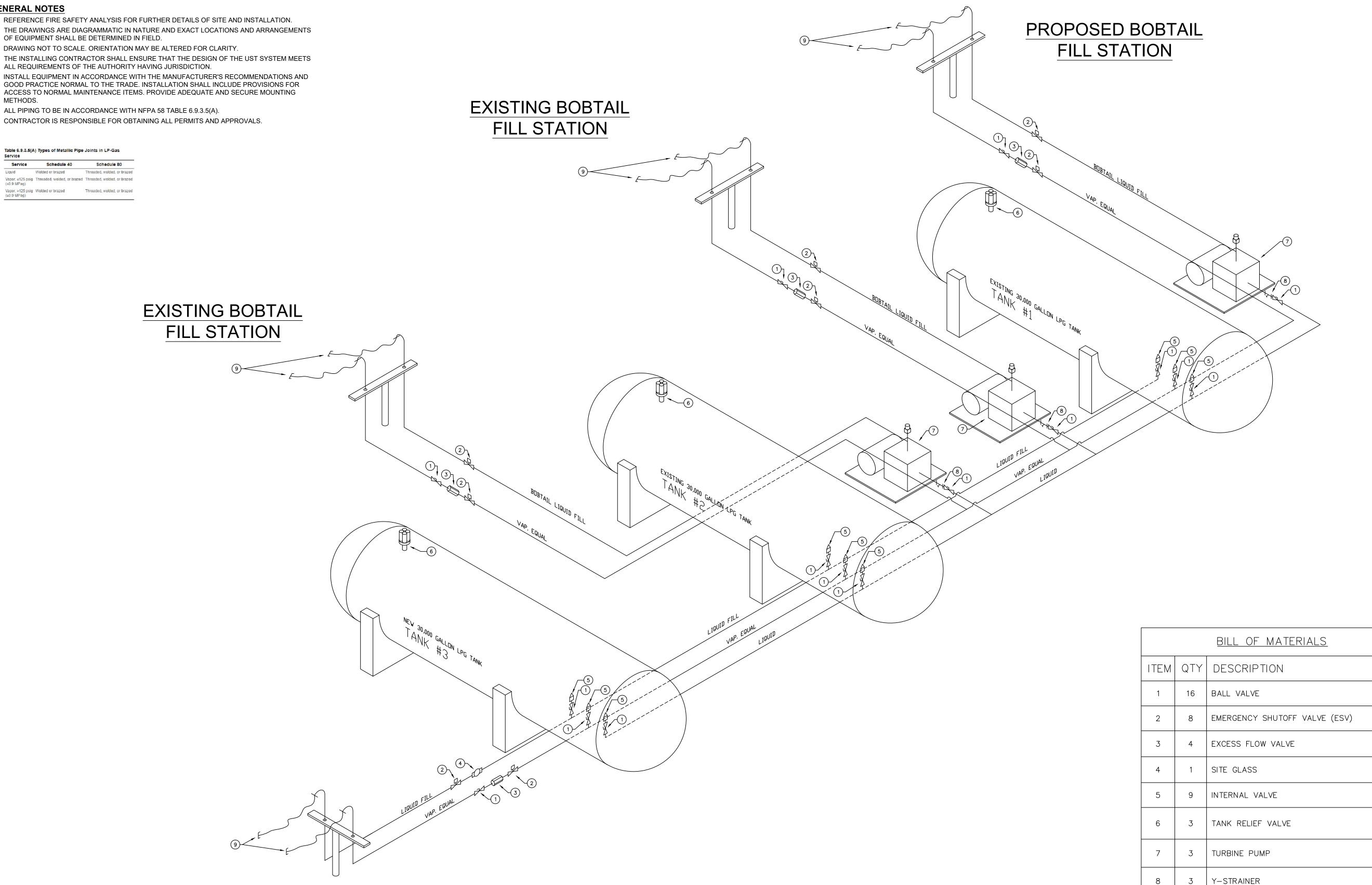
INSTALLATION OF ONE NEW 30,000 GALLON STORAGE TANK FOR STORAGE OF LIQUID PROPANE. TANKS TO BE CONSTRUCTED IN ACCORDANCE WITH NFPA 58.

GENERAL NOTES

- 1. REFERENCE FIRE SAFETY ANALYSIS FOR FURTHER DETAILS OF SITE AND INSTALLATION.
- 2. THE DRAWINGS ARE DIAGRAMMATIC IN NATURE AND EXACT LOCATIONS AND ARRANGEMENTS OF EQUIPMENT SHALL BE DETERMINED IN FIELD.
- 3. DRAWING NOT TO SCALE. ORIENTATION MAY BE ALTERED FOR CLARITY.
- 4. THE INSTALLING CONTRACTOR SHALL ENSURE THAT THE DESIGN OF THE UST SYSTEM MEETS
- 5. INSTALL EQUIPMENT IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND GOOD PRACTICE NORMAL TO THE TRADE. INSTALLATION SHALL INCLUDE PROVISIONS FOR ACCESS TO NORMAL MAINTENANCE ITEMS. PROVIDE ADEQUATE AND SECURE MOUNTING
- 6. ALL PIPING TO BE IN ACCORDANCE WITH NFPA 58 TABLE 6.9.3.5(A).
- 7. CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL PERMITS AND APPROVALS.

Table 6.9.3.5(A) Types of Metallic Pipe Joints in LP-Gas

Service	Schedule 40	Schedule 80
Liquid	Welded or brazed	Threaded, welded, or brazed
Vapor, ≤125 psig (≤0.9 MPag)	Threaded, welded, or brazed	Threaded, welded, or brazed
Vapor, ≥125 psig (≥0.9 MPag)	Welded or brazed	Threaded, welded, or brazed



EXISTING STANCHION TO BE RELOCATED

PROPOSED PIPING SCHEMATIC - N.T.S





ISSUED FOR: **PERMIT**

REVISIONS				
NO.	DESCRIPTION	DATE		
	I	ı		

CROWN ENERGY SUTTON, MA

PROPANE SYSTEM SCHEMATIC

CHECKED BY: GVR 03/23/2023

SHEET NO.:

8 ACME FITTINGS

FX-001

SHEET 1 OF 1