



To:	Matthew C. Benoit – Town of Douglas Community Development Director Jennifer Hager – Town of Sutton Planning & Economic Development Director
Cc:	Wayne Belec – Land Design Collaborative Timothy Flynn – Flynn Build & Develop
From:	Courtney E. Sudak, PE – Tetra Tech
Date:	June 2, 2022
Subject:	Peak Season Traffic Volume Assessment Proposed Sutton Douglas Development Sutton & Douglas, MA

Tetra Tech has conducted an evaluation of peak season traffic volume conditions at the six study area intersections previously evaluated in the September 2021 Traffic Impact and Access Study (TIAS) previously prepared by Tetra Tech for the proposed Sutton Douglas Development (the "Project"). The TIAS included an analysis of existing and future (with and without the Project) intersection operations at the study intersections based on turning movement counts collected in July 2021. As reported in the TIAS, seasonal traffic volume adjustment factors published by the Massachusetts Department of Transportation (MassDOT) indicate that July is an above average travel month. However, the region was still experiencing some impacts to typical traffic volume conditions even though travel and occupancy restrictions associated with the COVID-19 pandemic were no longer in place in Massachusetts at the time the counts were conducted. Furthermore, during the local planning board hearings for the project, residents indicated that the study area roadways have historically experienced a measurable amount of traffic associated with several area campgrounds that may not have been in operation or not fully occupied in July 2021 due to the pandemic. This evaluation provides estimates for traffic associated with these seasonal recreational uses and includes an assessment of intersection operations during this peak season condition.

This memorandum documents our findings.

## AREA RECREATIONAL USE TRIP GENERATION

Based on consultation with Town of Sutton Planning staff, the following recreational uses in the area may impact traffic at the study area intersections:

- Kings Family Campground located at 24 Holt Road in Sutton (114 camp sites)
- Sutton Falls Campground located at 90 Manchaug Road in Sutton (104 camp sites)
- Blueberry Island located at Whitin Reservoir in Douglas (1 camp site)
- Lake Manchaug Camping located at 70 Oak Street in Douglas (217 camp sites)
- Breezy Picnic Grounds & Water Slides located at 520 NW Main Street in Douglas (2.49 acres)
- Old Holbrook Place Campground located at 114 Manchaug Road in Sutton

It is our understanding that Old Holbrook Place Campground has been sold and will not be reopening as a campground. Therefore, traffic associated with Old Holbrook Place is assumed to be negligible for this analysis. Traffic associated with the other five recreational uses was estimated based on industry standard trip rates published by the Institute of Transportation Engineers (ITE) in *Trip Generation Manual*, 11<sup>th</sup> Edition (2021) using the most closely related land uses. The trip generation calculations are provided in Attachment A.

The majority of traffic associated with these sites is anticipated to use the regional highway system. There are several travel paths between the five properties and the regional roadway network. However, all traffic associated with these uses was assumed to use Duval Road to present a conservative analysis.

## PEAK SEASON TRAFFIC VOLUMES

The baseline traffic volumes were based on turning movement count data collected in July 2021 during the weekday morning (7AM to 9AM) and weekday evening (4PM to 6PM) peak periods as part of the TIAS. Seasonal adjustment factors published by the Massachusetts Department of Transportation (MassDOT) indicate that July is an above-average travel month (approximately 8 percent higher than average season conditions). As a conservative measure, the observed traffic volumes were not adjusted (reduced) to represent average season conditions.

MassDOT data for the week the traffic volumes were collected in July 2021 indicated that traffic at count locations nearest the site (Station ID AET06 Charlton and Station ID ASET07 Hopkinton) were approximately 1 percent to 10 percent lower than the same week in July 2019. The Town's traffic peer review consultant, MDM Transportation Consultants, Inc. (MDM) stated in their April 5, 2022 comment letter that traffic volume data for the area in 2020 (when COVID-19 travel and occupancy permits were in place) were 20 percent lower when compared to 2017 (pre-pandemic). As a conservative measure, the observed traffic volumes were adjusted (increased) by 20 percent to account for general COVID-19 related traffic volume fluctuations at the study intersections.

A comparison of the peak season 2021 Existing Condition peak hour traffic volumes and the Existing Conditions volumes reported in the September 2021 TIAS for the project is provided in Table 1. A Traffic Projection Model detailing the traffic volume assumptions from the observed (raw) traffic volume count data through the development of the peak season 2028 Build (with project) traffic volumes is provided in Attachment B.

Table 1 2021 Existing Condition Peak Hour Traffic Volume Comparison

				ection Volume per hour)		
		AM Peak Hour			PM Peak Hour	
Intersection	2021 TIAS <sup>1</sup>	Peak Season <sup>2</sup>	Net Change	2021 TIAS	Peak Season	Net Change
Torrey Road/ Duval Road	31	177	+146	66	254	+188
Mumford Road/ Duval Road	39	186	+147	57	244	+187
Birch Street/ Fairfax Way	24	88	+64	52	119	+67
Birch Street/ Belvoir Avenue	21	85	+64	46	112	+66

<sup>1) 2021</sup> Existing Condition peak hour traffic volumes presented in *Transportation Impact and Access Study, Proposed Residential Development, 61 Duval Road, Sutton, Massachusetts*, prepared by Tetra Tech, dated September 2021.

<sup>2)</sup> July 2021 peak hour traffic volumes adjusted for general traffic volume fluctuations due to the COVID-19 pandemic (20 percent increase) and traffic associated with area recreational properties.

## PEAK SEASON CAPACITY ANALYSES

Tetra Tech conducted capacity analyses at the six study intersections for the future 2028 Build (with project) peak season peak hour traffic volumes described above. The peak season capacity analyses indicate that the study intersections are expected to operate well below capacity with minimal delay at LOS B or better operations during the weekday morning and weekday evening peak hours under future conditions with the project in place. Therefore, ample capacity is available at the study area roadways and intersections to support the proposed residential project. Capacity analysis worksheets are provided in Attachment C.

## CONCLUSIONS

Supplemental capacity analyses were prepared at the six study intersections previously evaluated in the September 2021 TIAS prepared for the project to assess intersection operations during peak season conditions. The peak season peak hour traffic volumes were estimated and include a 20 percent increase to the observed data to account for general COVID-19 traffic volume fluctuations, traffic associated with several area recreational uses that may not have been in operation or were not fully occupied at the time the TIAS traffic counts were collected in July 2021 and the future year growth assumptions used in the TIAS. The supplemental capacity analyses conducted for the peak season traffic volume conditions indicate that the study intersections are expected to operate well below capacity with minimal delay at LOS B or better operations during the weekday morning and weekday evening peak hours under future conditions with the project in place. Therefore, ample capacity is available at the study area roadways and intersections to support the proposed residential project.

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Attachment A
Trip Generation Calculations

# Trip Generation Calculations ITE *Trip Generation, 11th Edition* Average Trip Rates (September 2021) Area Campgrounds/Water Park

#### **BREEZY PICNIC GROUNDS & WATER SLIDES**

Land Use Code 411 - Public	Park					Size:	2.49	Acres
Time Period	R <sup>2</sup> Value	Use Equation or Rate?	Equation	Rate	Percent Enter	ln	Out	Total
Saturday Peak Hour		Rate		22.92	58%	33	24	57

Source: Trip Generation, Eleventh Edition, (Institute of Transportation Engineers, 2021).

NOTE: LUC 482 (Water Slide Park) does not have any weekday data. Therefore, the Saturday peak hour data was used for both the weekday morning and weekday evening peak hours. The similar land use for LUC 411 (Public Park) results in negligible trips during the weekday peak hour. Therefore, it is anticipated that use of the Water Slide Park Saturday peak hour data is conservative.

#### KINGS CAMPGROUND

Land Use Code 416 - Campo	round/RV	Park				Size:	114	Occupied Campsites
Time Period	R <sup>2</sup> Value	Use Equation or Rate?	Equation	Rate	Percent Enter	ln	Out	Total
AM Street Peak Hour	0.96	Equation	T = 0.16(X) + 2.93	0.21	36%	8	13	21
PM Street Peak Hour	0.72	Rate	Ln(T)=.71ln(x)-0.06	0.27	65%	20	11	31

Source: Trip Generation, Eleventh Edition, (Institute of Transportation Engineers, 2021).

#### SUTTON FALLS CAMPGROUND

Land Use Code 416 - Camp	ground/RV	Park				Size:	104	Occupied Campsites
Time Period	R <sup>2</sup> Value	Use Equation	Equation	Rate	Percent Enter	In	Out	Total
AM Street Peak Hour	0.96		T = 0.16(X) + 2.93	0.21	36%	7	13	
PM Street Peak Hour	0.72	Rate	Ln(T)=.71ln(x)-0.06	0.27	65%	18	10	28

Source: Trip Generation, Eleventh Edition, (Institute of Transportation Engineers, 2021).

#### **BLUEBERRY ISLAND CAMPGROUND**

Land Use Code 416 - Cam	ground/RV	Park				Size:	1	Occupied Campsites
Time Period	R <sup>2</sup> Value	Use Equation or Rate?	Equation	Rate	Percent Enter	In	Out	Total
AM Street Peak Hour	0.96	Equation	T = 0.16(X) + 2.93	0.21	36%	1	2	3
PM Street Peak Hour	0.72	Rate	Ln(T)=.71ln(x)-0.06	0.27	65%	0	0	0

Source: Trip Generation, Eleventh Edition, (Institute of Transportation Engineers, 2021).

#### LAKE MANCHAUG CAMPGROUND

Land Use Code 416 - Cam	pground/RV	Park				Size:	217	Occupied Campsites
Time Period	R <sup>2</sup> Value	Use Equation or Rate?	Equation	Rate	Percent Enter	ln	Out	Total
AM Street Peak Hour	0.96	Equation	T = 0.16(X) + 2.93	0.21	36%	14	24	38
PM Street Peak Hour	0.72	Rate	Ln(T)=.71ln(x)-0.06	0.27	65%	38	21	59

Source: Trip Generation, Eleventh Edition, (Institute of Transportation Engineers, 2021).

Attachment B
Traffic Projection Model

## TRAFFIC PROJECTION MODEL

		1		1				-1								1				ı	1										1			
				Seasonal						Breezy	2021										Seasona	1					Breezy	2021				1		1 1
				ly		Kings	Sutton			Picnic	Existing										ly		Kings	Sutton	Blueberry	Lake	Picnic	Existing		2028 No		1		1
		July 2021 AM				Family	Falls		Manchaug			2028 Grown		2028 No			·		July 2021 PM				Family	Falls			Grounds &		2028 Grown	Build PM		1 .		2028 Build
		Peak Hour As- Counted	Traffic Volumes	AM Peak Hour	Existing AM Peak	nd AM	nd AM	ou Campgrou nd AM		Water Slides AM	Hour Traffic	AM Peak	2028 Grown AM	Build AM			AM Peak	AM Peak Hour	Peak Hour As- Counted	Volumes	c PM Peak Hour	Existing PM Peak	nd PM	nd PM	nd PM	nd PM	Water Slides PM	Hour Traffic	PM Peak 2028 Hour Traffic Grown PM	Peak Hour			oite Trips PM Peak	PM Peak Hour
		Volumes	Adjusted for		Hour			ur Peak Hou				Volumes	Peak Hour		Entering	Exiting	Hour	Traffic	Volumes	Adjusted fo					Peak Hour		Peak Hour		Volumes Peak Hou		Entering		Hour	Traffic
Torrey Road & Duval Road		(Unadjusted)	COVID-19	Volumes	Balancing		Trips	Trips	Trips	Trips		(Unadjusted			Distribution				(Unadjusted)				Trips	Trips	Trips	Trips	Trips		(Unadjusted) Balancing			Distribution (A		Volumes
7:45 AM - 8:00 AM	NBT	10	12	12	_						12	14		14		10%	2	16	7	8	8							8	9	9		10%	1	10
4:00 PM - 5:00 PM	NBR	4	5	5				2	1	24	31	36		36	10%		0	36	8	10	10				0		24	34	39	39	10%		2	41
Seasonal Adjustment Factor	SBL	7	8	8		13	13		24		58	67		67	20%		1	68	10	12	12		11	10		21		54	62	62	20%		4	66
1.00	SBT	3	4	4							4	5		5	10%		1	6	22	26	26							26	30	30	10%		2	32
	WBL	3	4	4				1		33	38	44		44		10%	2	46	16	19	19				0		33	52	60	60		10%	1	61
Annual Growth Rate	WBR	4	5	5		8	7		14		34	39		39		20%	4	43	3	4	4		20	18		38		80	92	92		20%	3	95
2.0%																																		
Mumford Road & Duval Road																																		
7:00 AM - 8:00 AM	NBL	1	1	1		8	7		14		30	34		34			0	34	3	4	4		20	18		38		80	92	92			0	92
4:00 PM - 5:00 PM	NBT	18	22	22							22	25		25			0	25	12	14	14							14	16	16			0	16
Design Horizon (Years)	SBT	3	4	4							4	5		5			0	5	10	12	12	1						12	14	14			0	14
7.00	SBR	4	5	5				1	ļ	33	39	45		45	50%		4	49	18	22	22	1			0		33	55	63	63	50%		11	74
	EBL	11	13	13				2		24	39	45		45		50%	9	54	11	13	13	<u> </u>			0		24	37	43	43		50%	6	49
Growth Factor	EBR	2	2	2		13	13		24		52	60		60			0	60	3	4	4		11	10		21		46	53	53		$\longrightarrow$	0	53
1.149																																$\leftarrow \rightarrow$		
Birch Street & Fairfax Way																																<del></del>		
8:00 AM - 9:00 AM	NBT	12	14	14				2		24	40	46		46	10%	10%	2	48	11	13	13				0		24	37	43	43	10%	10%	3	46
4:00 PM - 5:00 PM	NBR	1	1	1							1	1		1			0	1	1	1	1 -							1	1	1			0	1
2015 111 1 1 1	SBL	2	2	2					ļ		2	2	1	2	100/	100/	0	2	6	7	7	1			_			/	8	8	1001	100/	0	8
COVID Adjustment Factor 1,20	SBT	5 1	6	6				1	ļ	33	40	46	1	46	10%	10%	3	49	30 0	36 0	36 0	1			0		33	69	79	79 0	10%		3	82 0
1.20	WBR	3	4	4		-		+	1		4	5	+	5			0	5	4	5	5	+						- 0	6	6		+	0	6
	WBR	3	4	4		-		+	1		4	5	+	5			U	5	4	5	5	+						5	0	0		+	U	
Birch Street & Belvoir Avenue	-			1		1		+					+							1	+	+										+		-
8:00 AM - 9:00 AM	NBT	10	12	12				2	<b> </b>	24	38	44	-	44	10%	+	0	44	10	12	12	+			0		24	36	41	41	10%	+	2	43
4:00 PM - 5:00 PM	NBR	1	1	1		1			1	2.7	1	1	+	1	10%		1	2	2	2	2	1					2-7	2	2	2	10%	$\leftarrow$	2	4
4.30 T W - 0.00 T W	SBL	0	0	0		1		+	1		0	0	+	0	10%		1	1	5	6	6	1						6	7	7	10%		2	9
	SBT	6	7	7		1		1		33	41	47		47	1070	10%	2	49	25	30	30	1			0		33	63	72	72	1070		1	73
	WBL	1	1	1							1	1		1		10%	2	3	1	1	1							1	1	1		10%	1	2
	WBR	3	4	4							4	5		5		10%	2	7	3	4	4	1 1						4	5	5		10%	1	6
																						1												
Proposed Subdivision Roadway & Duval Ro	ad																															<u> </u>		
7:30 AM - 8:30 AM	NBL	0	0	0							0	0		0		30%	6	6	0	0	0							0	0	0		30%	4	4
4:00 PM - 5:00 PM	NBR	0	0	0							0	0		0		50%	9	9	0	0	0							0	0	0		50%	6	6
	EBT	12	14	14		13	13	2	24	24	90	103		103			0	103	15	18	18		11	10	0	21	24	84	96	96		ــــــــــــــــــــــــــــــــــــــ	0	96
	EBR	0	0	0							0	0		0	30%		1	1	0	0	0							0	0	0	30%		6	6
	WBL	0	0	0							0	0		0	50%		4	4	0	0	0							0	0	0	50%		11	11
	WBT	9	11	11		8	7	1	14	33	74	85		85			0	85	20	24	24	1	20	18	0	38	33	133	153	153			0	153
									ļ								ļ					1												
Forest Street & Proposed Subdivision Road									ļ								ļ																	
8:00 AM - 9:00 AM	NBT	1	1	1					ļ		1	1		1			0	1	3	4	4							4	5	5			0	5
4:00 PM - 5:00 PM	NBR	0	0	0				_			0	0		0			0	0	0	0	0	<u> </u>					ļ	0	0	0		$\longrightarrow$	0	0
	SBL	0	0	0							0	0	1	0	20%	1	2	2	0	0	0	1						0	0	0	20%		4	4
	SBT	1	1	1							1	1	1	1		1	0	1	1	1	1	1						1	1	1			0	1
	WBL	0	0	0							0	0	1	0			0	0	0	0	0	1						0	0	0		<del></del>	0	0
	WBR	U	0	0					l		0	0	1	0		20%	4	4	U	0	U			<u> </u>	<u> </u>		1	U	0	0		20%	2	2

Attachment C
Capacity Analysis Worksheets

Intersection						
Int Delay, s/veh	7					
		MED	Not	NEE	051	057
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		<b>^</b>		•	र्
Traffic Vol, veh/h	46	43	16	36	68	6
Future Vol, veh/h	46	43	16	36	68	6
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	58	58	58	58	50	50
Heavy Vehicles, %	0	0	0	25	14	0
Mvmt Flow	79	74	28	62	136	12
Major/Minor N	/linor1	N	Major1		Major2	
Conflicting Flow All	343	59	0	0	90	0
Stage 1	59	-	U	U	-	-
Stage 2	284	-	_	_	_	_
	6.4	6.2	-	-	4.24	-
Critical Hdwy	5.4	0.2	_	_	4.24	
Critical Hdwy Stg 1			-	-		-
Critical Hdwy Stg 2	5.4	-	-	-	- 206	-
Follow-up Hdwy	3.5	3.3	-	-	2.326	-
Pot Cap-1 Maneuver	657	1012	-	-	1433	-
Stage 1	969	-	-	-	-	-
Stage 2	769	-	-	-	-	-
Platoon blocked, %	-0.1	1010	-	-	1 100	-
Mov Cap-1 Maneuver	594	1012	-	-	1433	-
Mov Cap-2 Maneuver	594	-	-	-	-	-
Stage 1	969	-	-	-	-	-
Stage 2	695	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	11.1		0		7.1	
HCM LOS	В		U		7.1	
TIOWI LOG	U					
Minor Lane/Major Mvmt	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	742	1433	-
HCM Lane V/C Ratio		-	-	0.207	0.095	-
HCM Control Delay (s)		-	-	11.1	7.8	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh)		-	-	8.0	0.3	-

Intersection						
Int Delay, s/veh	5.5					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	22	0.4	4	ĵ.	40
Traffic Vol, veh/h	54	60	34	25	5	49
Future Vol, veh/h	54	60	34	25	5	49
Conflicting Peds, #/hr	0	0	0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	81	81	59	59	58	58
Heavy Vehicles, %	0	0	0	8	0	6
Mvmt Flow	67	74	58	42	9	84
Major/Minor M	inor?		Major1	A	laier?	
	inor2		Major1		/lajor2	^
Conflicting Flow All	209	51	93	0	-	0
Stage 1	51	-	-	-	-	-
Stage 2	158	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	784	1023	1514	-	-	-
Stage 1	977	-	-	-	-	-
Stage 2	875	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	753	1023	1514	-	-	-
Mov Cap-2 Maneuver	753	-	-	-	-	-
Stage 1	939	-	-	-	-	-
Stage 2	875	_	_	-	-	-
U =						
A I			. LID		0.0	
Approach	EB		NB		SB	
HCM Control Delay, s	9.9		4.3		0	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBL	NRT	EBLn1	SBT	SBR
		1514	-		-	אופט
( 'anacity (yah/h)		0.038		0.161	_	-
Capacity (veh/h)			-	U. 10 I	-	-
HCM Lane V/C Ratio						
HCM Lane V/C Ratio HCM Control Delay (s)		7.5	0	9.9	-	-
HCM Lane V/C Ratio					- -	-

Intersection						
Int Delay, s/veh	0.4					
		WED	NET	NDD	ODL	ODT
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	_	ĵ.			4
Traffic Vol, veh/h	1	5	48	1	2	49
Future Vol, veh/h	1	5	48	1	2	49
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	65	65	44	44
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	1	5	74	2	5	111
	inor1		//ajor1		Major2	
Conflicting Flow All	196	75	0	0	76	0
Stage 1	75	-	-	-	-	-
Stage 2	121	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	797	992	-	-	1536	-
Stage 1	953	-	-	-	-	-
Stage 2	909	-	_	-	-	-
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	795	992	_	_	1536	_
Mov Cap-2 Maneuver	795	-	_	_	-	_
Stage 1	953					-
•	906	_	_	-	_	-
Stage 2	300	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.8		0		0.3	
HCM LOS	A					
= 0 0	, ,					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_		953	1536	
HCM Lane V/C Ratio		<u>-</u>		0.006		_
HCM Control Delay (s)		_	_	8.8	7.4	0
HCM Lane LOS			-	Α	7.4 A	A
HCM 95th %tile Q(veh)		-		0	0	
now your wille Q(ven)		-	-	U	U	-

Intersection						
Int Delay, s/veh	0.6					
		WED	NOT	NDD	ODI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	-	ĵ.	0	4	<b></b> €
Traffic Vol, veh/h	3	7	44	2	1	49
Future Vol, veh/h	3	7	44	2	1	49
Conflicting Peds, #/hr	0	0	0	0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	69	69	50	50
Heavy Vehicles, %	0	0	0	0	0	4
Mvmt Flow	3	7	64	3	2	98
Major/Minor N	1inor1	N	Major1	N	Major2	
Conflicting Flow All	168	66	0	0	67	0
Stage 1	66	-	_	-	-	-
Stage 2	102	-	-	_	-	-
Critical Hdwy	6.4	6.2	_	-	4.1	_
Critical Hdwy Stg 1	5.4	-	-	_	_	-
Critical Hdwy Stg 2	5.4	_	_	-	_	_
Follow-up Hdwy	3.5	3.3	-	_	2.2	-
Pot Cap-1 Maneuver	827	1003	-	_	1547	_
Stage 1	962	-	-	_	_	-
Stage 2	927	_	_	-	_	_
Platoon blocked, %			-	_		_
Mov Cap-1 Maneuver	826	1003	_	-	1547	-
Mov Cap-2 Maneuver	826	-	_	_	-	_
Stage 1	962	_	_	_	_	_
Stage 2	926	_	_	_	_	_
Clago 2	020					
Approach	WB		NB		SB	
HCM Control Delay, s	8.9		0		0.1	
HCM LOS	Α					
Minor Lane/Major Mvmt	•	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_			1547	
HCM Lane V/C Ratio		<u>-</u>		0.011		_
HCM Control Delay (s)		_	_	8.9	7.3	0
HCM Lane LOS		_	_	Α	Α.	A
HCM 95th %tile Q(veh)		_	_	0	0	-
				J	- 0	

Intersection						
Int Delay, s/veh	0.6					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	¥	
Traffic Vol, veh/h	103	1	4	85	6	9
Future Vol, veh/h	103	1	4	85	6	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	_	None	-	None	-	None
Storage Length	-	-	-	_	0	-
Veh in Median Storag	e.# 0	-	-	0	0	-
Grade, %	0	_	_	0	0	-
Peak Hour Factor	60	60	38	38	92	92
Heavy Vehicles, %	8	2	2	0	2	2
Mvmt Flow	172	2	11	224	7	10
Majay/Minay	Maiart		Maia#2		Aire a red	
	Major1		Major2		Minor1	470
Conflicting Flow All	0	0	174	0	419	173
Stage 1	-	-	-	-	173	-
Stage 2	-	-	- 4.40	-	246	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-		2.218		3.518	
Pot Cap-1 Maneuver	-	-	1403	-	591	871
Stage 1	-	-	-	-	857	-
Stage 2	-	-	-	-	795	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver		-	1403	-	586	871
Mov Cap-2 Maneuver	-	-	-	-	586	-
Stage 1	-	-	-	-	857	-
Stage 2	-	-	-	-	788	-
Approach	EB		WB		NB	
HCM Control Delay, s			0.3		10.1	
HCM LOS	U		0.0		В	
TIOW LOS					U	
Minor Lane/Major Mvr	nt 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		729	-	-	1403	-
HCM Lane V/C Ratio		0.022	-	-	0.008	-
HCM Control Delay (s	s)	10.1	-	-	7.6	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh	1)	0.1	-	-	0	-

Intersection						
Int Delay, s/veh	4.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		<b>1</b>			4
Traffic Vol, veh/h	0	4	1	0	2	1
Future Vol, veh/h	0	4	1	0	2	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	92	92	25	25	25	25
Heavy Vehicles, %	2	2	0	2	2	0
Mvmt Flow	0	4	4	0	8	4
IVIVIIIL I IUW	U	7	4	- 0	U	7
Major/Minor N	Minor1	N	Major1		Major2	
Conflicting Flow All	24	4	0	0	4	0
Stage 1	4	-	-	-	-	-
Stage 2	20	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	992	1080	_	-	1618	-
Stage 1	1019	-	-	_	_	_
Stage 2	1003	-	-	-	-	_
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	987	1080	_	_	1618	_
Mov Cap-1 Maneuver	987	-	_	_	-	_
Stage 1	1019		-	-		<u>-</u>
Stage 2	998	-	-	_	_	_
Slaye 2	330	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.3		0		4.8	
HCM LOS	Α					
Minor Long /Mairy M	4	NDT	MDD	MDL 4	CDI	CDT
Minor Lane/Major Mvm	I	NBT		WBLn1	SBL	SBT
		-				-
		-	-			-
		-	-			0
		-	-			Α
HCM 95th %tile Q(veh)		-	-	0	0	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)		-	- - -	1080 0.004 8.3 A	1618 0.005 7.2 A 0	C A

Intersection						
Int Delay, s/veh	7.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	WDL	WDR		INDIX	SDL	
Lane Configurations		0E	<b>♣</b>	11	ee.	<b>र्दी</b> 32
Traffic Vol, veh/h	61	95	10	41	66	
Future Vol, veh/h	61	95	10	41	66	32
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	68	68	75	75	89	89
Heavy Vehicles, %	6	0	0	0	0	0
Mvmt Flow	90	140	13	55	74	36
Major/Minor I	Minor1	N	//ajor1	N	Major2	
Conflicting Flow All	225	41	0	0	68	0
Stage 1	41	_	-	_	-	_
Stage 2	184	_	_	_	_	_
Critical Hdwy	6.46	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.46	-	_	_	-	_
Critical Hdwy Stg 2	5.46	_	_	_	_	_
Follow-up Hdwy	3.554	3.3	_	_	2.2	_
Pot Cap-1 Maneuver	754	1036	_	_	1546	_
Stage 1	971	-	_	_	-	_
Stage 2	838	_	_	_	_	_
Platoon blocked, %	000		_	_		_
Mov Cap-1 Maneuver	717	1036	_	_	1546	_
Mov Cap-1 Maneuver	717	1030	_		1340	
Stage 1	971	-	-	<u>-</u>	<u>-</u>	-
Stage 2	797	_	_	-	_	-
Slaye 2	191	-	-	-	<u>-</u>	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.5		0		5	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NRRV	VBLn1	SBL	SBT
Capacity (veh/h)		1101	-		1546	051
HCM Lane V/C Ratio		-	-		0.048	_
HCM Control Delay (s)		<u>-</u>	_		7.4	0
HCM Lane LOS		-	_	10.5 B	7.4 A	A
HCM 95th %tile Q(veh	١	-	-	1	0.2	- -
HOW JOHN JOHNE W(VEH	1	_	_	ı	0.2	_

Intersection						
Int Delay, s/veh	6.6					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	50	00	4	Þ	7.4
Traffic Vol, veh/h	49	53	92	16	14	74
Future Vol, veh/h	49	53	92	16	14	74
Conflicting Peds, #/hr	0	0	0	0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	70	70	47	47	70	70
Heavy Vehicles, %	0	0	0	8	0	6
Mvmt Flow	70	76	196	34	20	106
Major/Minor M	/linor2	N	/lajor1	N	/lajor2	
Conflicting Flow All	499	73	126	0	-	0
Stage 1	73	-	-	-	_	-
Stage 2	426	<u>-</u>	_		_	_
Critical Hdwy	6.4	6.2	4.1	_	_	
Critical Hdwy Stg 1	5.4	- 0.2	7.1	_	_	_
Critical Hdwy Stg 2	5.4		_	-	_	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
	535	995	1473	<u>-</u>		_
Pot Cap-1 Maneuver	955		14/3	-	-	-
Stage 1		-	-	-	-	-
Stage 2	663	-	-	-	-	-
Platoon blocked, %	100	005	4.470	-	-	-
Mov Cap-1 Maneuver	462	995	1473	-	-	-
Mov Cap-2 Maneuver	462	-	-	-	-	-
Stage 1	825	-	-	-	-	-
Stage 2	663	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	12.3		6.7		0	
HCM LOS	12.3 B		0.1		U	
TIOWI LOO	U					
Minor Lane/Major Mvmt	t	NBL	NBTI	EBLn1	SBT	SBR
Capacity (veh/h)		1473	-	640	-	-
HCM Lane V/C Ratio		0.133	-	0.228	-	-
HCM Control Delay (s)		7.8	0	12.3	-	-
HCM Lane LOS		Α	Α	В	-	-
		0.5	_	0.9	_	_
HCM 95th %tile Q(veh)		0.0		0.0		

Intersection						
Int Delay, s/veh	1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL	אטוי		NON	JDL	<u> </u>
Traffic Vol, veh/h	<b>"</b>	6	<b>♣</b>	1	8	<b>€</b> 1 82
			46			82
Future Vol, veh/h	0	6		1	8	
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	50	50	75	75	75	75
Heavy Vehicles, %	0	0	0	0	0	3
Mvmt Flow	0	12	61	1	11	109
Major/Minor	Aire a 4		Anic of		Ania TO	
	/linor1		Major1		Major2	
Conflicting Flow All	193	62	0	0	62	0
Stage 1	62	-	-	-	-	-
Stage 2	131	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	800	1009	_	-	1554	-
Stage 1	966	-	-	-	-	-
Stage 2	900	-	_	-	_	-
Platoon blocked, %	000		_	_		_
Mov Cap-1 Maneuver	794	1009	_	-	1554	-
Mov Cap-2 Maneuver	794	1009		_	1004	_
•	966		-	-		-
Stage 1		-	-	-	-	-
Stage 2	893	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.6		0		0.7	
HCM LOS	Α		U		0.1	
TIOWI LOG						
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	_	1009	1554	-
HCM Lane V/C Ratio		-		0.012		-
HCM Control Delay (s)		-	_	8.6	7.3	0
HCM Lane LOS		_	_	A	A	A
HCM 95th %tile Q(veh)		_	_	0	0	-
				- 0	- 0	

Intersection						
Int Delay, s/veh	1.4					
		WDD	NDT	NDD	CDI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	^	<b>^</b>	4	^	<u>ન</u>
Traffic Vol, veh/h	2	6	43	4	9	73
Future Vol, veh/h	2	6	43	4	9	73
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	33	33	65	65	63	63
Heavy Vehicles, %	0	0	0	0	0	4
Mvmt Flow	6	18	66	6	14	116
Major/Minor M	1inor1	N	Major1	ı	Major2	
Conflicting Flow All	213	69	0	0	72	0
Stage 1	69	-	-	-	-	-
Stage 2	144	<u>-</u>	_	_	_	_
Critical Hdwy	6.4	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.4	- 0.2	_	_	7.1	_
Critical Hdwy Stg 2	5.4	-	_	<u>-</u>		
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	780	1000	-		1541	-
•	959	1000	-	-	1541	-
Stage 1			-	-	-	
Stage 2	888	-	-	-	-	-
Platoon blocked, %	770	1000	_	-	1511	-
Mov Cap-1 Maneuver	772	1000	-	-	1541	-
Mov Cap-2 Maneuver	772	-	-	-	-	-
Stage 1	959	-	-	-	-	-
Stage 2	879	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9		0		0.8	
HCM LOS	A		· ·		0.0	
TIOM EGO						
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-		1541	-
HCM Lane V/C Ratio		-	-	0.026		-
HCM Control Delay (s)		-	-	9	7.4	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh)		-	-	0.1	0	-

Intersection						
Int Delay, s/veh	0.6					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1→	LDIX	TTDL	<u>₩</u>	¥	וטוי
Traffic Vol, veh/h	96	6	11	153	4	6
Future Vol, veh/h	96	6	11	153	4	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		- Olop	None
Storage Length	_	-	_	-	0	-
Veh in Median Storage,	# 0	_	_	0	0	_
Grade, %	0	<u>-</u>	_	0	0	_
Peak Hour Factor	75	75	56	56	92	92
Heavy Vehicles, %	8	2	2	0	2	2
Mymt Flow	128	8	20	273	4	7
IVIVITIT FIOW	120	0	20	2/3	4	1
Major/Minor N	/lajor1	N	Major2	ľ	Minor1	
Conflicting Flow All	0	0	136	0	445	132
Stage 1	-	-	-	-	132	-
Stage 2	-	_	-	_	313	-
Critical Hdwy	_	_	4.12	-	6.42	6.22
Critical Hdwy Stg 1	_	_	_	_	5.42	_
Critical Hdwy Stg 2	-	_	-	_	5.42	_
Follow-up Hdwy	_	_	2.218	_	3.518	3.318
Pot Cap-1 Maneuver	_	_	1448	_	571	917
Stage 1	_	_	-	_	894	-
Stage 2	_	_	_	_	741	_
Platoon blocked, %	_	_		_	171	
Mov Cap-1 Maneuver	_		1448	_	562	917
Mov Cap-1 Maneuver		_	1440	-	562	917
	-	-			894	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	729	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.5		10	
HCM LOS	•				В	
N. 1 (0.1 ) 1		IDI 4		ED.5	14/5:	MOT
Minor Lane/Major Mvmt	1 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		732	-	-		-
HCM Lane V/C Ratio		0.015	-	-	0.014	-
HCM Control Delay (s)		10	-	-	7.5	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0	-	-	0	-

Intersection Int Delay, s/veh	3.8					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	•	ĵ.	•		र्न
Traffic Vol, veh/h	0	2	5	0	4	1
Future Vol, veh/h	0	2	5	0	4	1
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	38	38	25	25
Heavy Vehicles, %	2	2	0	2	2	0
Mvmt Flow	0	2	13	0	16	4
		_				
	Minor1		//ajor1		Major2	
Conflicting Flow All	49	13	0	0	13	0
Stage 1	13	-	-	-	-	-
Stage 2	36	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy		3.318	-	-	2.218	-
Pot Cap-1 Maneuver	960	1067	-	-	1606	-
Stage 1	1010	-	-	-	-	-
Stage 2	986	-	_	-	_	-
Platoon blocked, %	300		_	_		_
Mov Cap-1 Maneuver	950	1067	_	_	1606	_
Mov Cap-1 Maneuver	950	-	_	_	1000	_
•			-	-		
Stage 1	1010	-	-	-	-	-
Stage 2	976	-	-	-	-	-
Clage 2						
Clago 2						
			NB		SB	
Approach	WB		NB 0		SB 5.8	
Approach HCM Control Delay, s	WB 8.4		NB 0		SB 5.8	
Approach	WB					
Approach HCM Control Delay, s HCM LOS	WB 8.4 A	NRT	0	VRI n1	5.8	SRT
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvn	WB 8.4 A	NBT	0	VBLn1	5.8 SBL	SBT
Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvn Capacity (veh/h)	WB 8.4 A	-	0 NBRV	1067	5.8 SBL 1606	-
Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvn Capacity (veh/h) HCM Lane V/C Ratio	8.4 A	-	0 NBRV -	1067 0.002	5.8 SBL 1606 0.01	-
Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvn Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	8.4 A	- - -	NBRV - -	1067 0.002 8.4	5.8 SBL 1606 0.01 7.3	- - 0
Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvn Capacity (veh/h) HCM Lane V/C Ratio	8.4 A	-	0 NBRV -	1067 0.002	5.8 SBL 1606 0.01	-